Observed Structural Bridge Damage from M_w 7.8 Earthquake in Southern Turkiye (Turkey) and Model for Nonlinear Time-History Analysis

by

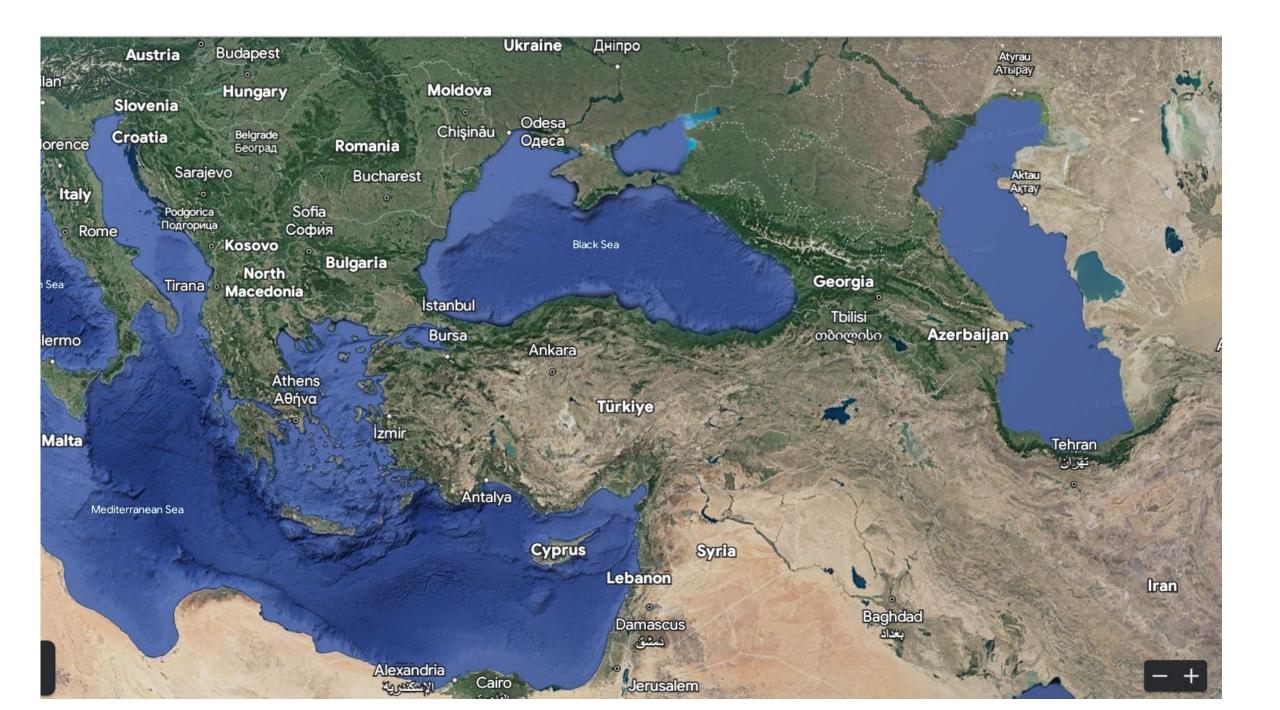
Prof. Robert K. Dowell, Ph.D., P.E. Director, Structural Engineering Laboratory

Department of Civil, Construction and Environmental Engineering College of Engineering San Diego State University (SDSU)

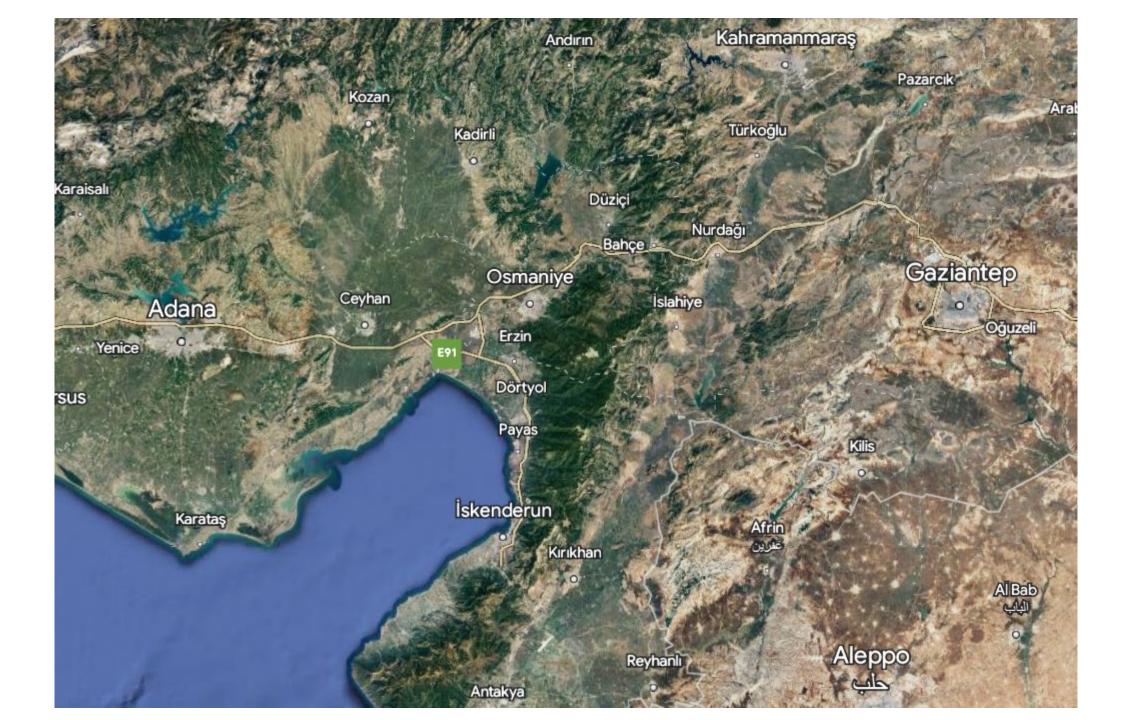
Structural Engineering Reconnaissance Team (Mountain Goats)

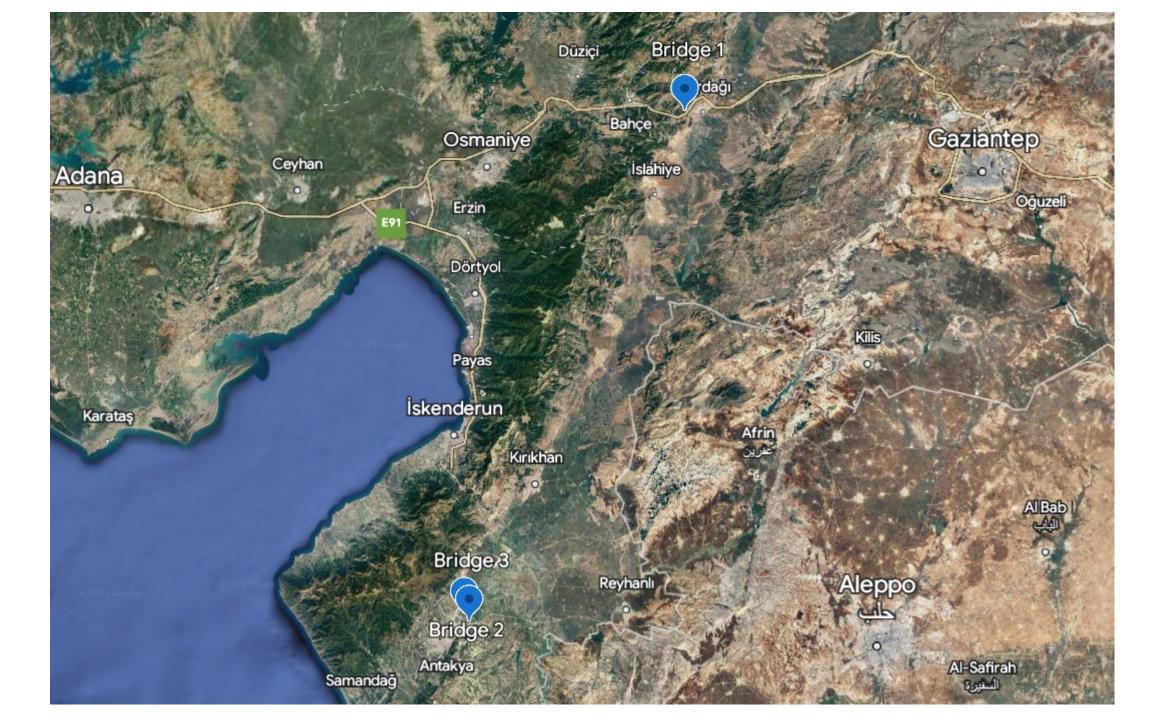
Robert Dowell, Gulen Ozkula, Ayse Hortacsu, Tunc Deniz Uludag, Jui-Liang Lin

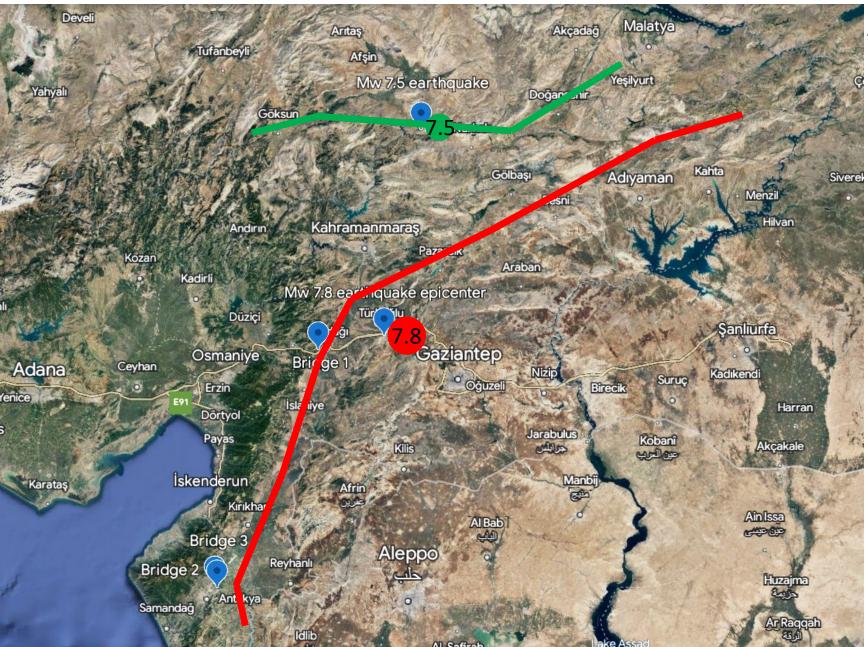










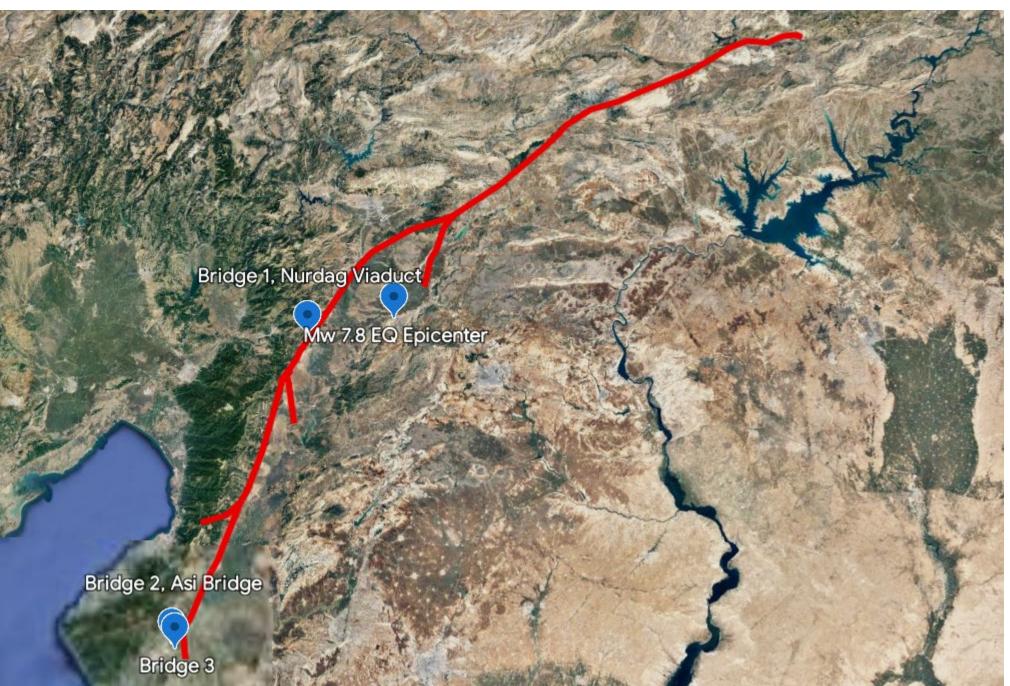


180 mile (290 km) Rupture Length for
M_w 7.8 earthquake (*M* 8.1/ Richter Scale)
Equivalent Distance of San Diego to

Santa Barbara in California or most of the length of Taiwan







180 mile (290 km) Rupture Length for M_w 7.8 earthquake

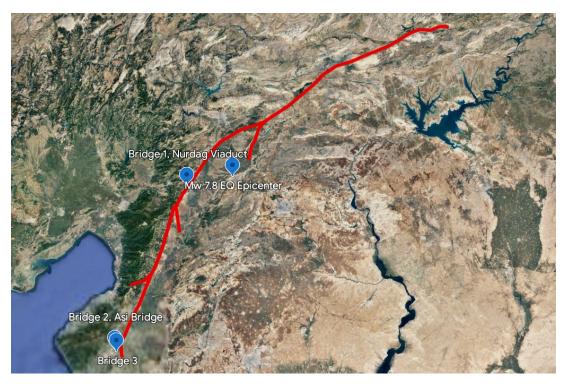
Bridge 1 – 0.05 miles (80 m) from the fault rupture

Bridge 2 – 2.5 miles (4.0 km) from the fault rupture

Bridge 3 – 2.0 miles (3.14 km) from the fault rupture

Intensity of Ground Shaking Follows the Fault Rupture Line for a Large Earthquake

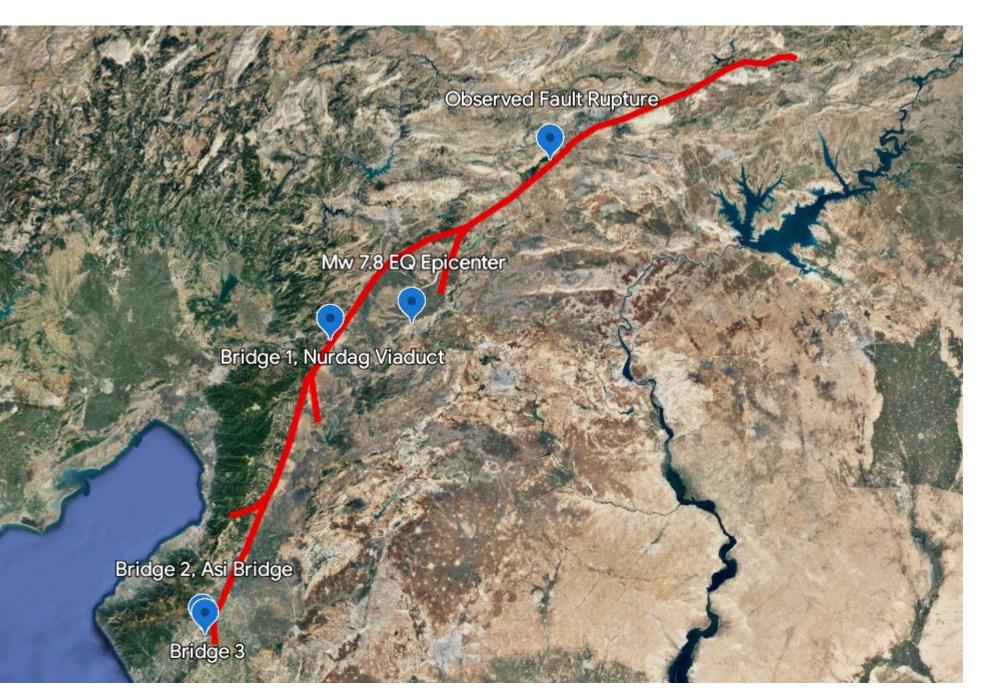
- Not concentric circles about the earthquake epicenter -



Fault Rupture (USGS)



Ground Shaking Intensity (USGS)



Observed versus Defined (USGS) Fault Rupture









2023 Turkiye Earthquake

Observed Fault Rupture

Six Days in the Field in Southern Turkiye, Team Mountain Goats

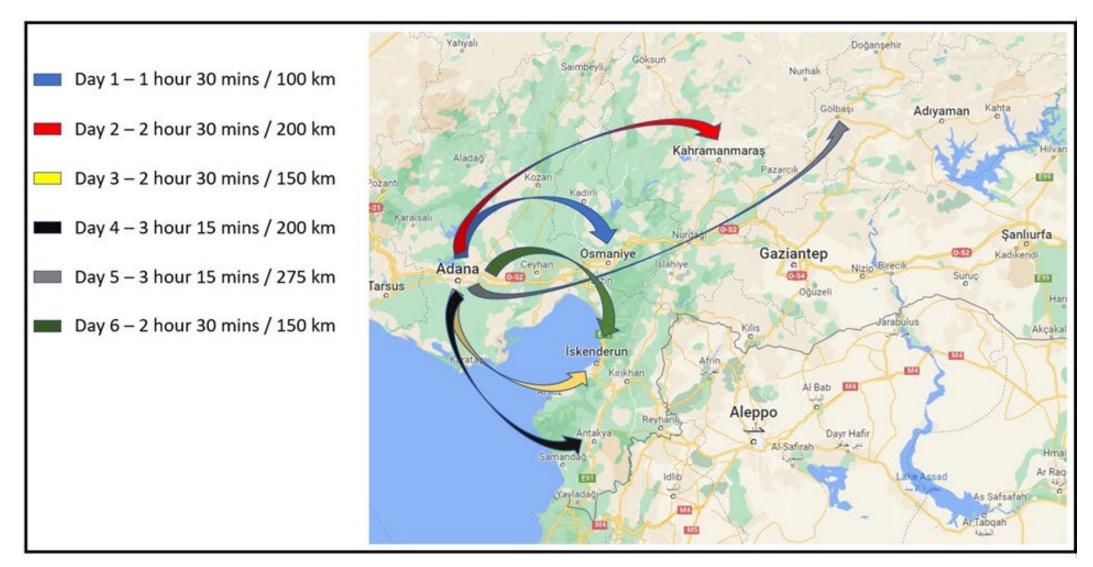


Image developed by team member Tunc Deniz Uludag

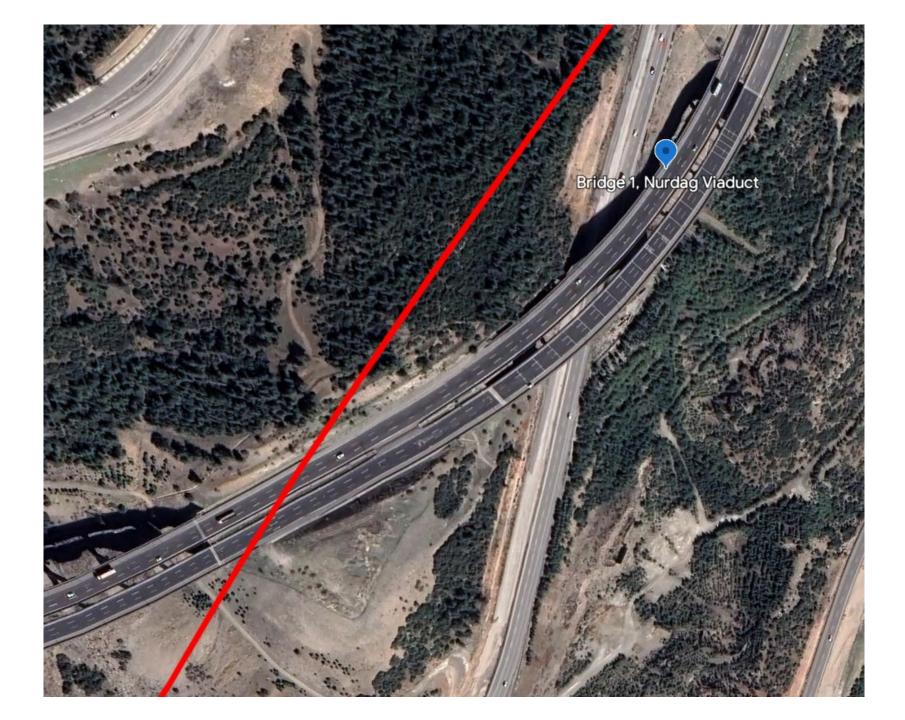




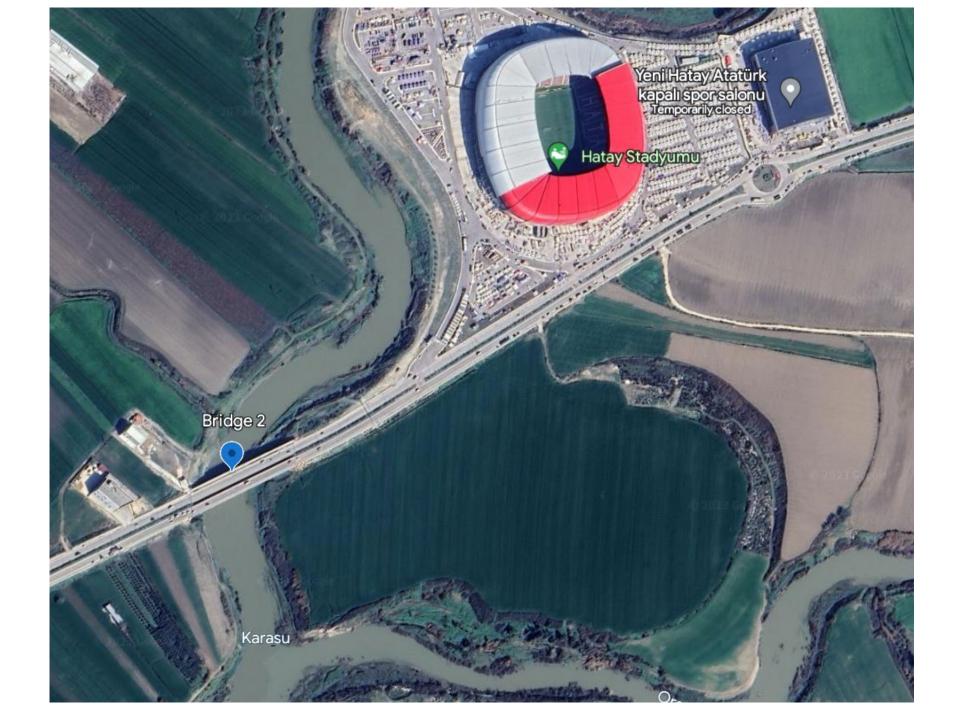
Fault Rupture



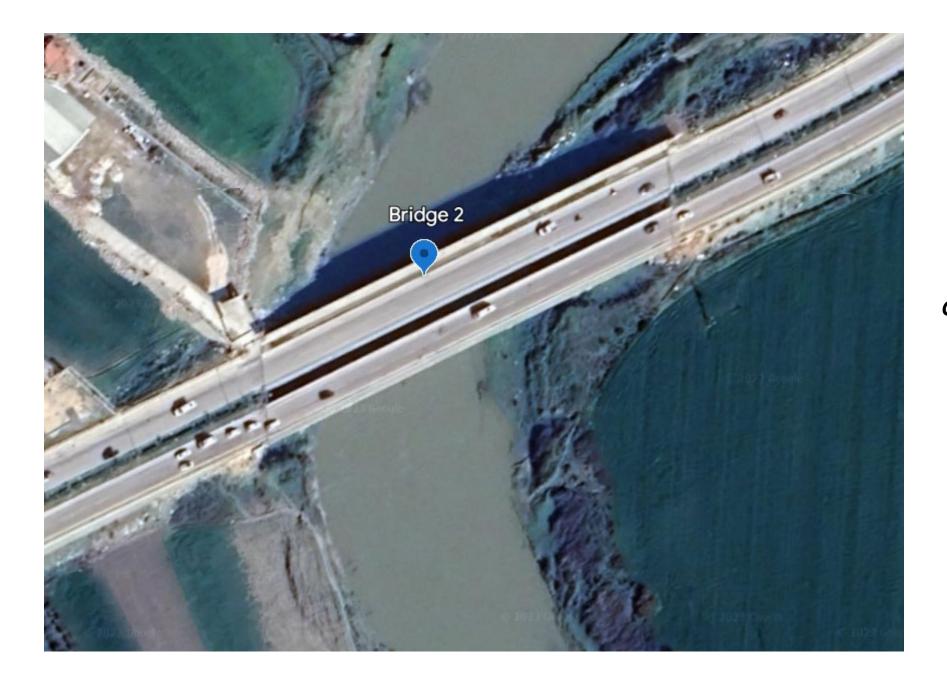
Fault Rupture



Fault Rupture



Asi Bridge

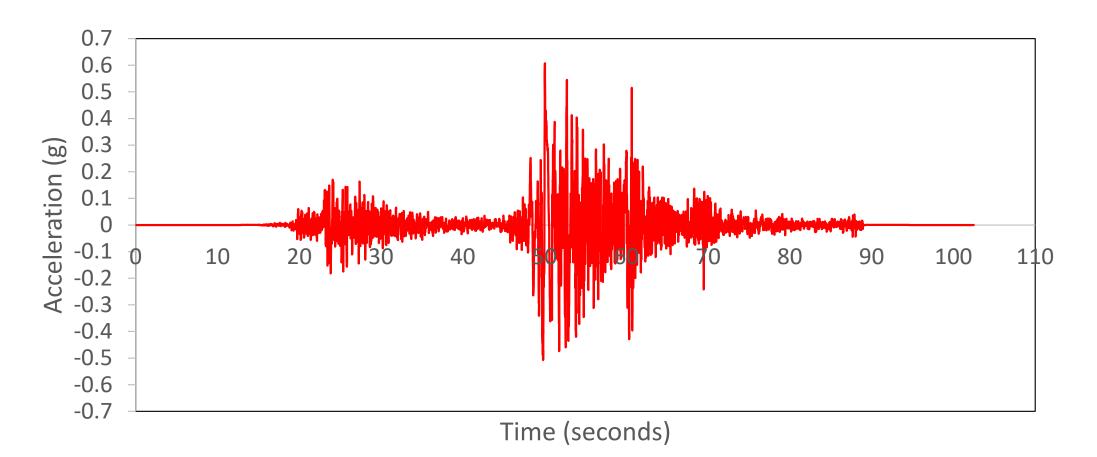




EW Motion for Station 2712, Closest to Bridge 1

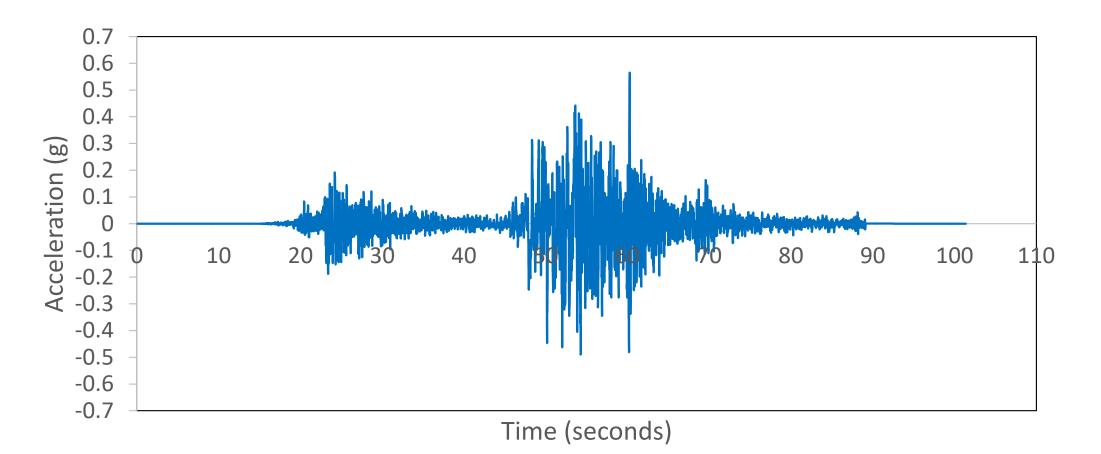
2.00 miles (3.21 km) from Bridge 1

PGA = 0.607 g



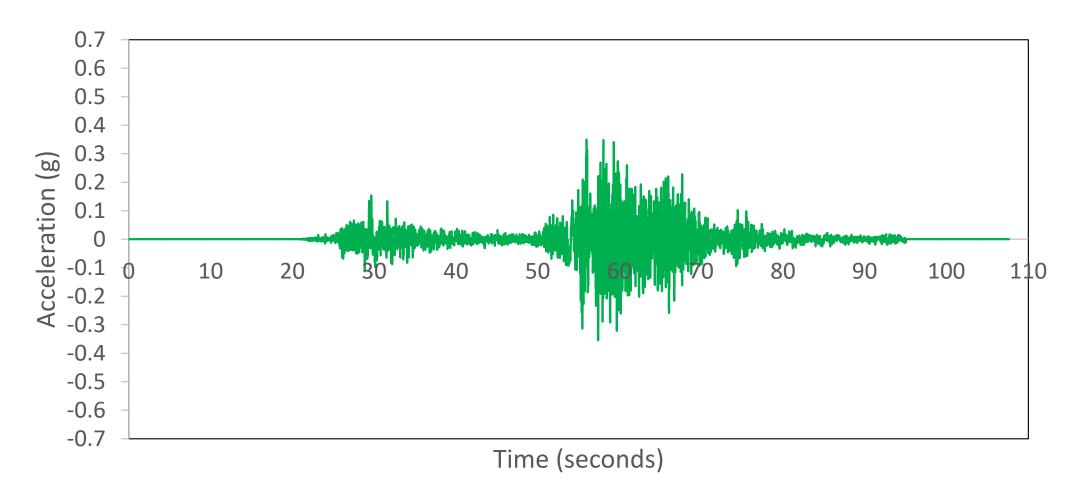
NS Motion for Station Closest to Bridge 1

PGA = 0.565 g

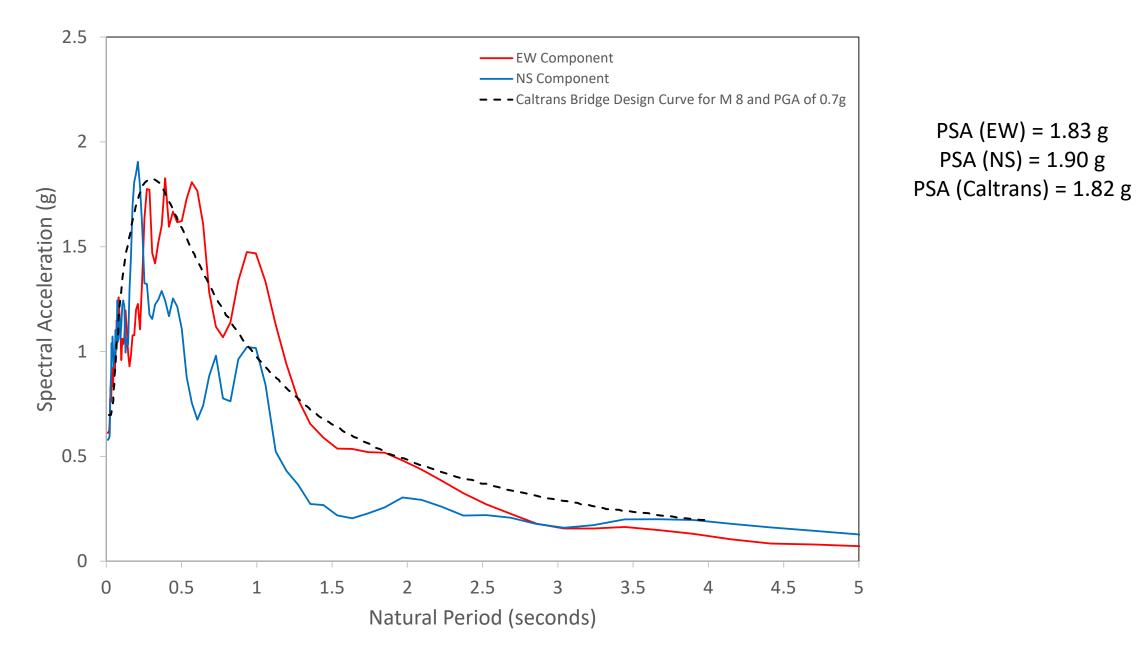


Vertical Motion for Station Closest to Bridge 1

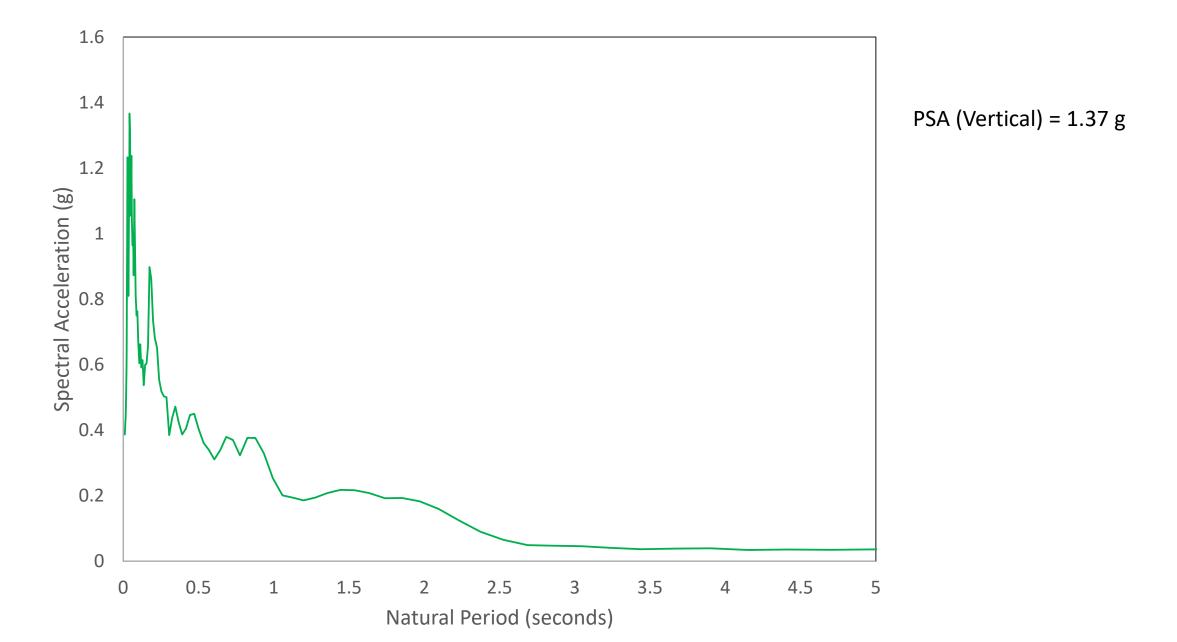
PGA = 0.354 g



EW and NS Spectra for Station Closest to Bridge 1



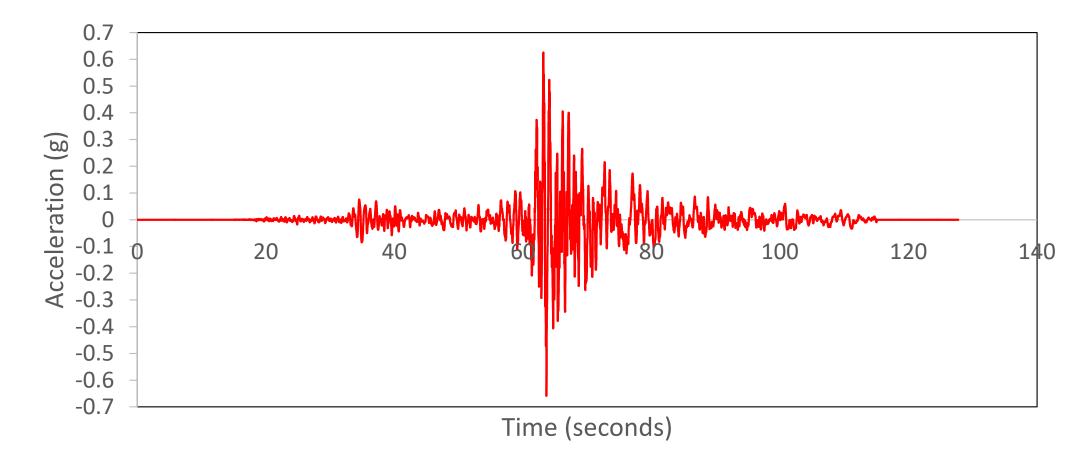
Vertical Spectra for Station Closest to Bridge 1



EW Motion for Station 3124, Closest to Bridges 2 and 3

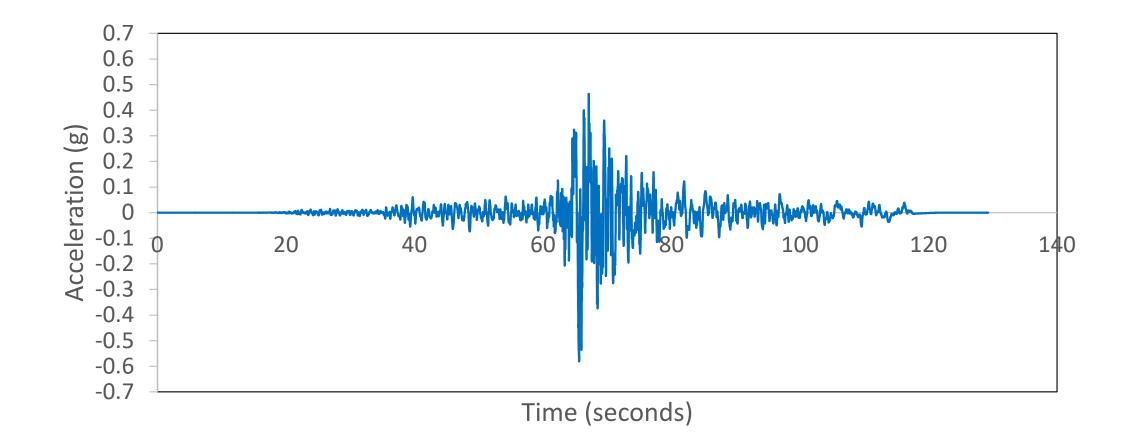
2.14 miles (3.44 km) from Bridge 2 and 2.33 miles (3.75 km) from Bridge 3

PGA = 0.659 g



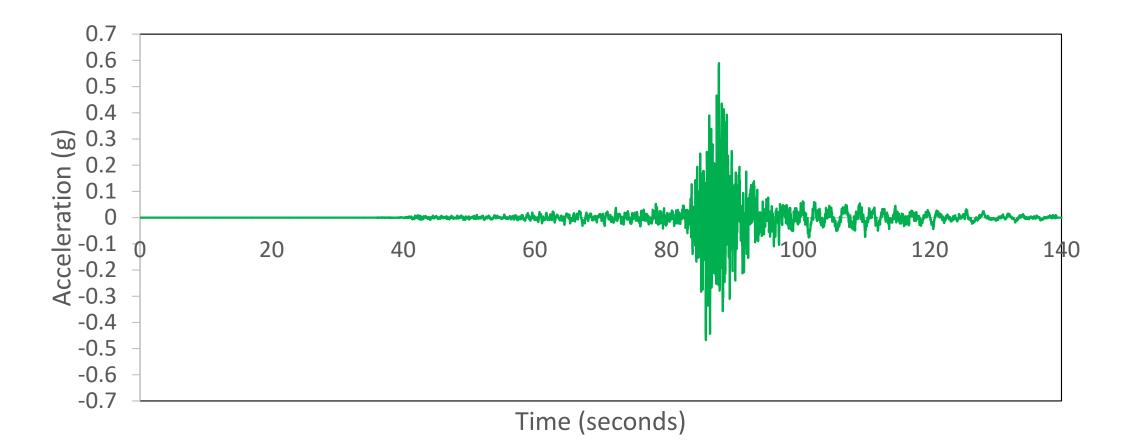
NS Motion for Station Closest to Bridges 2 and 3

PGA = 0.581 g

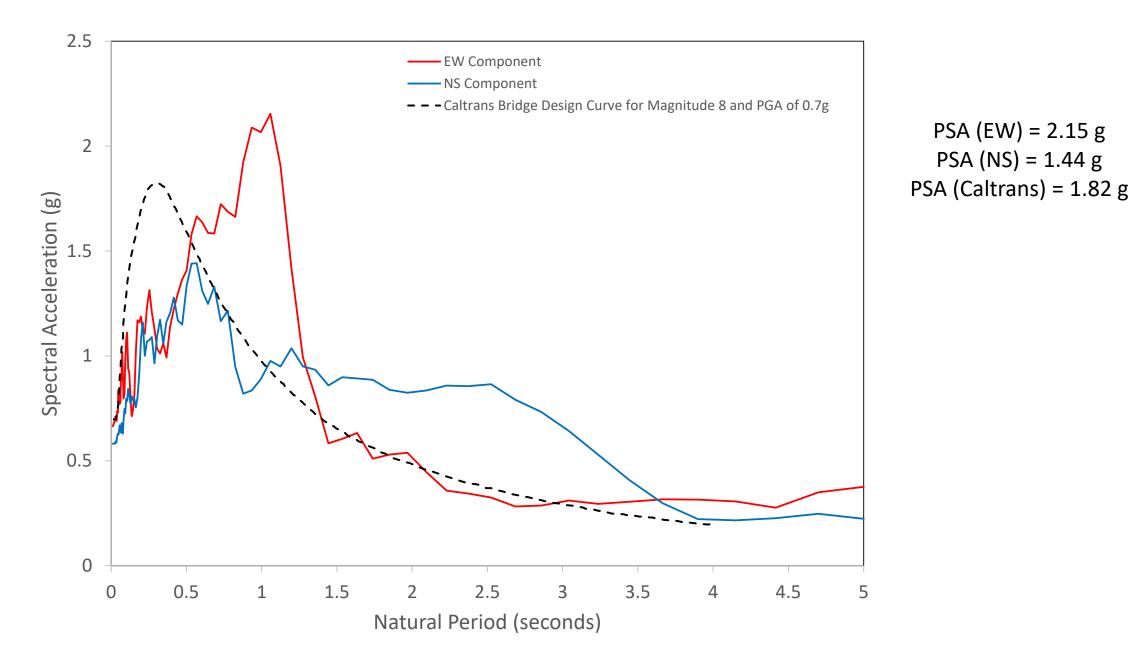


Vertical Motion for Station Closest to Bridges 2 and 3

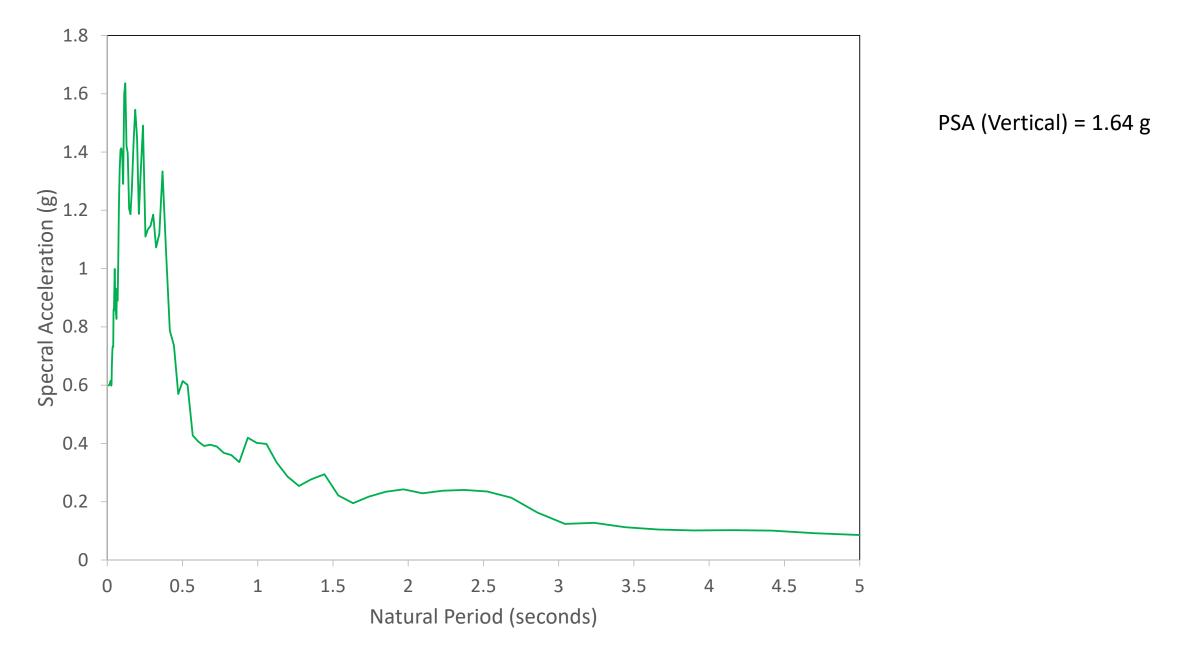
PGA = 0.589 g



EW and NS Spectra for Station Closest to Bridges 2 and 3



Vertical Spectra for Station Closest to Bridges 2 and 3



Bridge 1, 10' Diameter Cantilever RC Columns, about 80' Tall

(Nurdagi Viaduct)

18.1 miles (29.2 km) from the epicenter of the *Mw* 7.8 earthquake

260 feet (80 meters) from the fault rupture



Plastic hinge formed about 25% up the column height

Bridge 1, Looking Toward Abutments



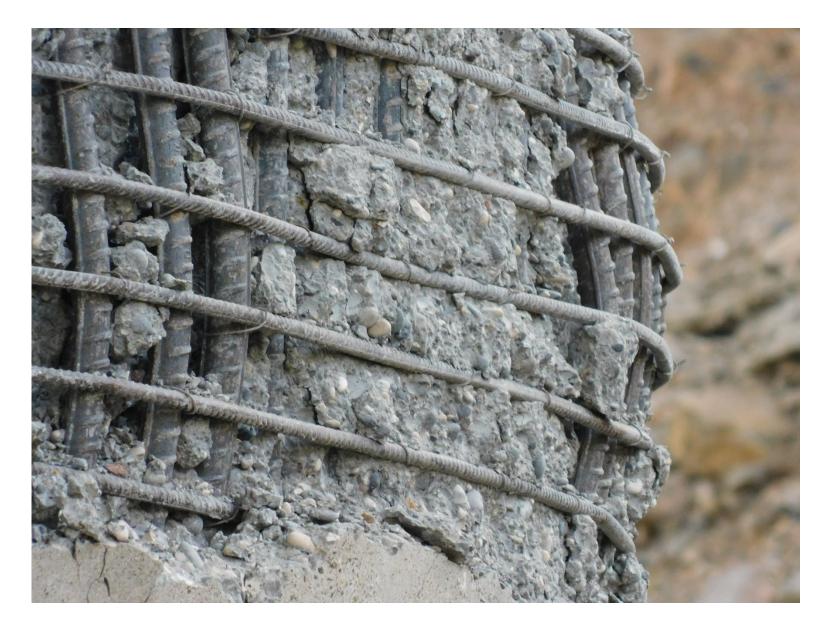
Bridge 1, Looking Toward Opposite Abutments



Bridge 1, Side View of Plastic Hinge



Bridge 1, Close-Up of one side of Plastic Hinge



Vertical Rebar Buckled, Transverse Rebar Yielded and Deformed

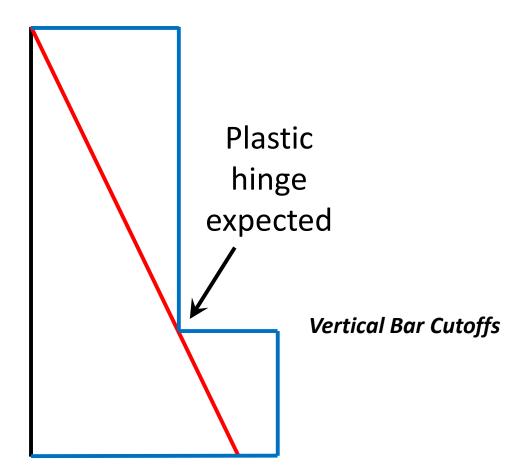
Bridge 1, Opposite side of Plastic Hinge, Spalled Concrete only



No Rebar Buckled, and Transverse Rebar not Deformed

Bridge 1, Plastic Hinge not at Base of Column

Moment CapacityMoment Demand



Bridge 1, Abutment Damage



Bridge 2, Overall View

(Asi Bridge)

80.8 miles (130 km) from the epicenter of the Mw 7.8 earthquake

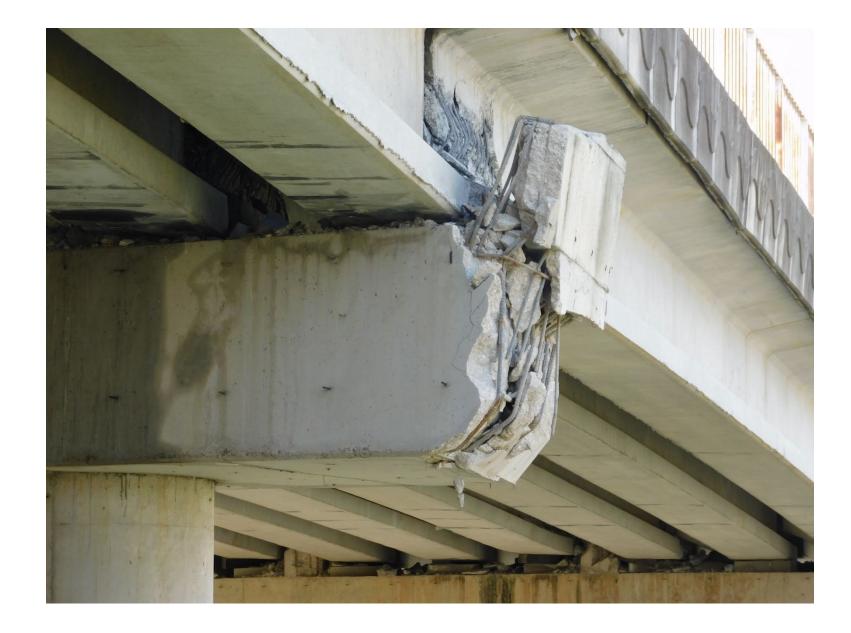
2.5 miles (4.0 km) from the fault rupture



Bridge 2, Exterior Shear Key Failures



Bridge 2, Close-Up of Exterior Shear Key Failure



Bridge 2, Exterior Shear Key Damage, Looking Along Bridge



Bridge 2, Rubber Bearing Pads on Ground



Bridge 2, End-of-Girder Damage, Exterior Girder



Bridge 2, no Concrete left at End-of-Girder, Exterior Girder



Bridge 2, Overall Exterior Girder Damage



Bridge 2, Prestress Stand from Precast Girder



Bridge 2, Close-Up of Prestress Strand



Bridge 2, Interior Girder Damage



Bridge 2, Close-Up of Interior Girder Damage



Bridge 2, Interior Girder Damage – no concrete left



Bridge 2, Damage and Twisting of Girder End



Bridge 2, Close-Up of Damage and Twisting of Girder End



Bridge 2, Bent Vertically/Horizontally Precast Girder



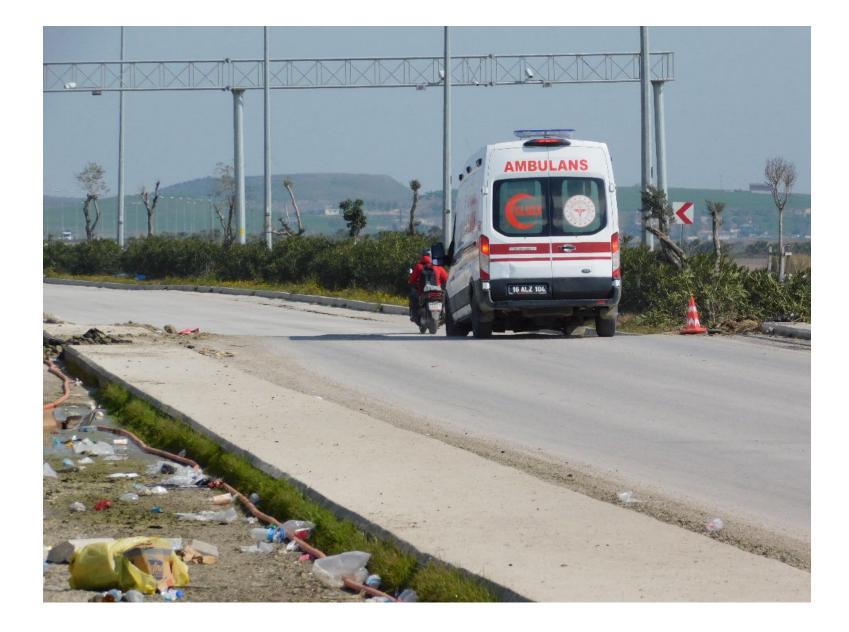
Bridge 2, Plastic Hinge at Base of Column in Weak Direction



Bridge 2, Approach Settlement



Bridge 2, Approach Settlement



Bridge 3, Side View

81.4 miles (131 km) from the epicenter of the Mw 7.8 earthquake

2.0 miles (3.14 km) from the fault rupture



Bridge 3, from Underneath



Bridge 3, End-of-Girder Damage



Bridge 3, Close-Up of End-of-Girder Damage



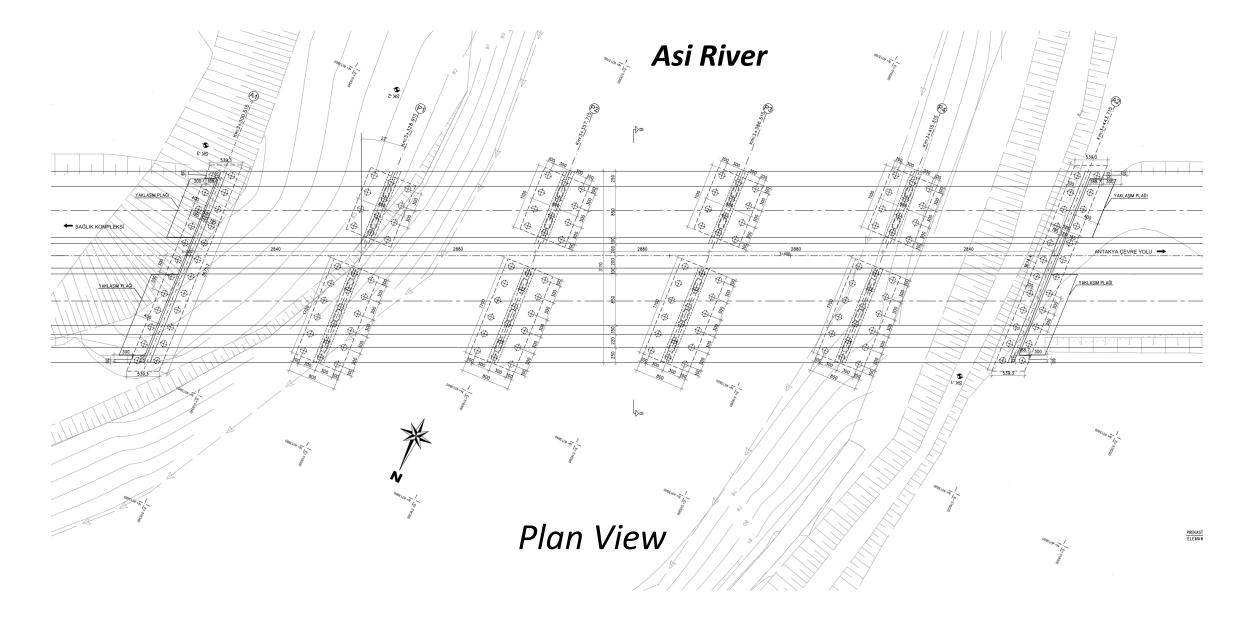
Bridge 3, Exterior Shear Key Damage

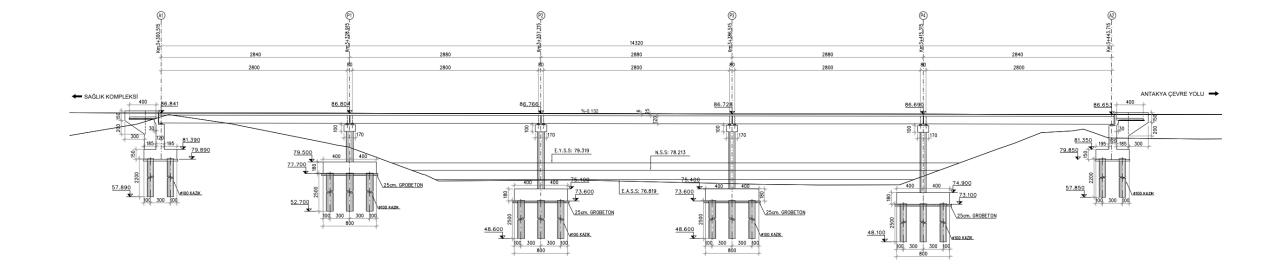


Bridge 3, Interior Shear Key Failure



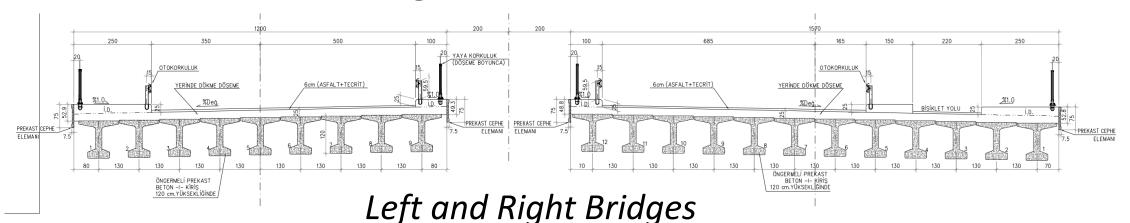
Bridge 2 (Asi Bridge), Bridge Plans and Analysis Model

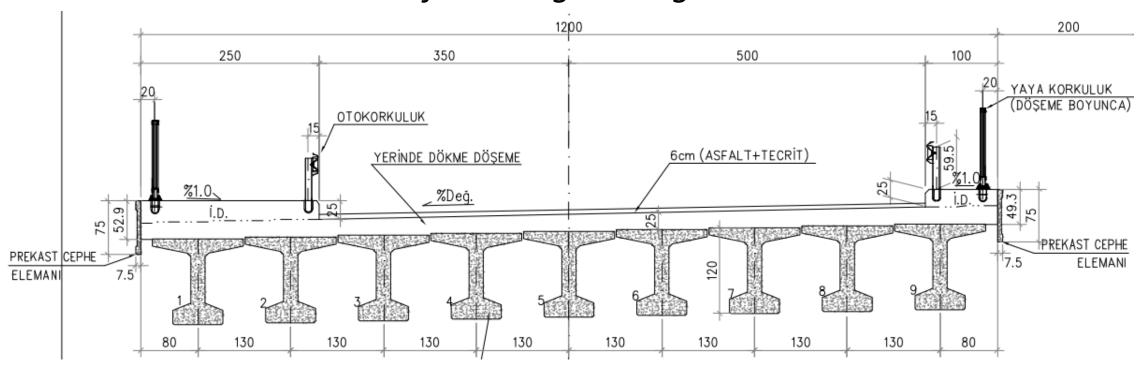




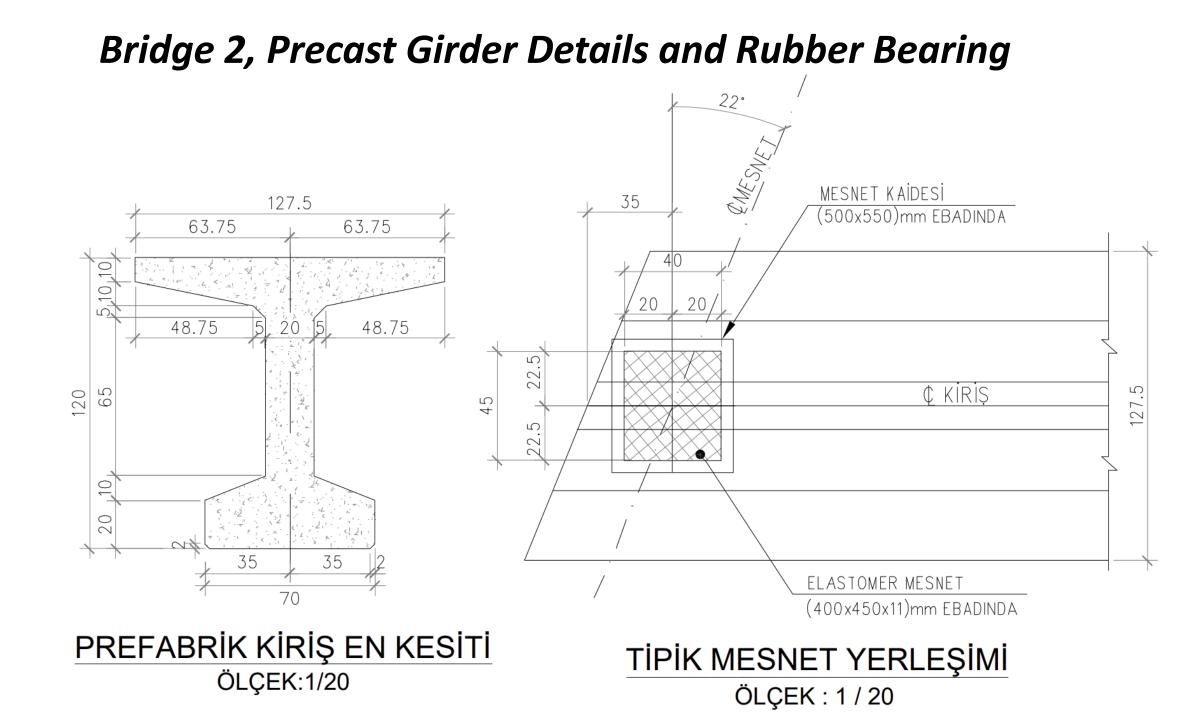
Elevation View

Bridge 2, Cross-Section

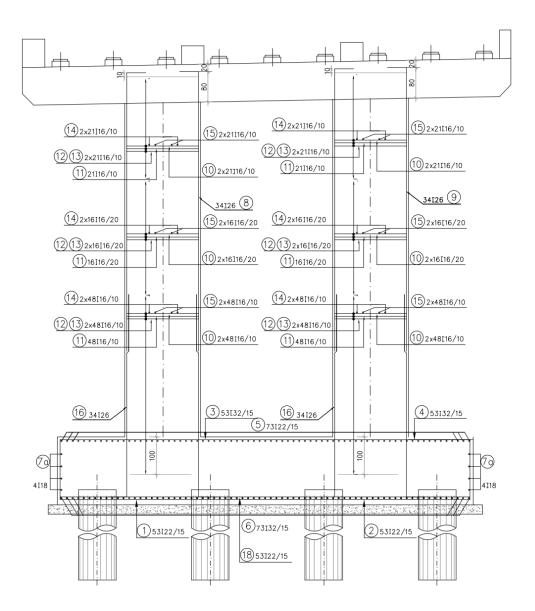




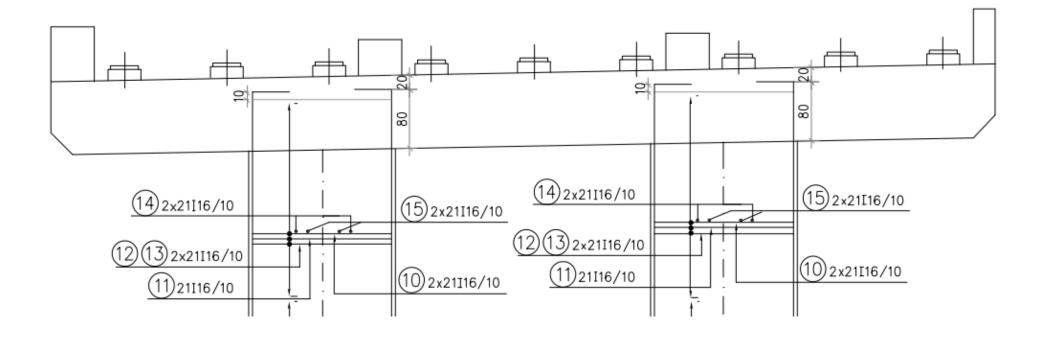
Left Bridge (Analysis Model is for Left Bridge only)



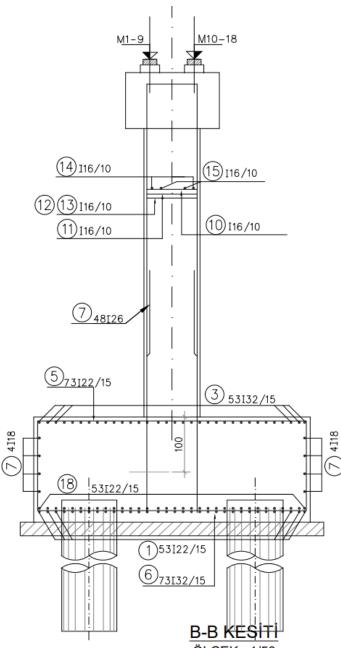
Bridge 2, Overall Bent Details



Bridge 2, Close-Up of Upper Bent Details



Bridge 2, Side view of Bent and Girder Supports



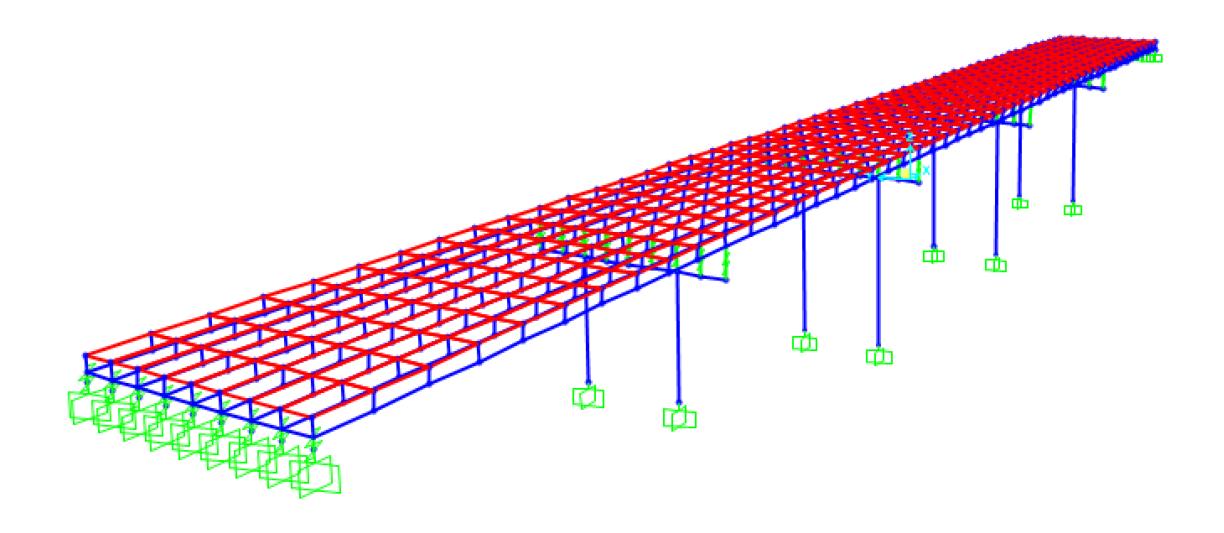
Bridge Model (in progress) – Beam Elements for Precast Girders and Columns

- Nonlinear springs at all girder ends
- Allow girder uplift (compression but no tension force), and sliding in both horizontal directions at supports
- Nonlinear moment-rotation springs at columns ends (Pivot Hysteresis Model)
- Nonlinear springs for impact at shear keys and abutments

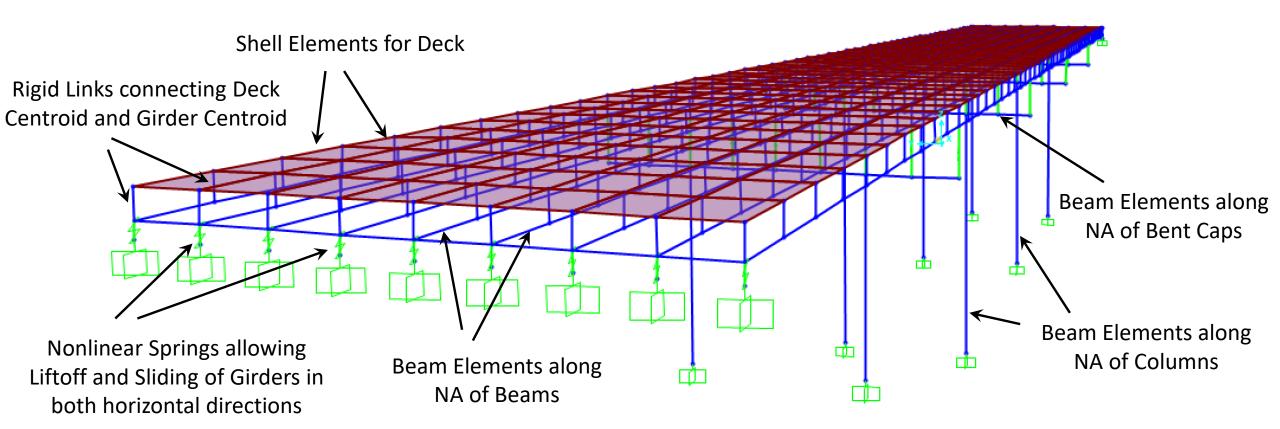
(Left Bridge)

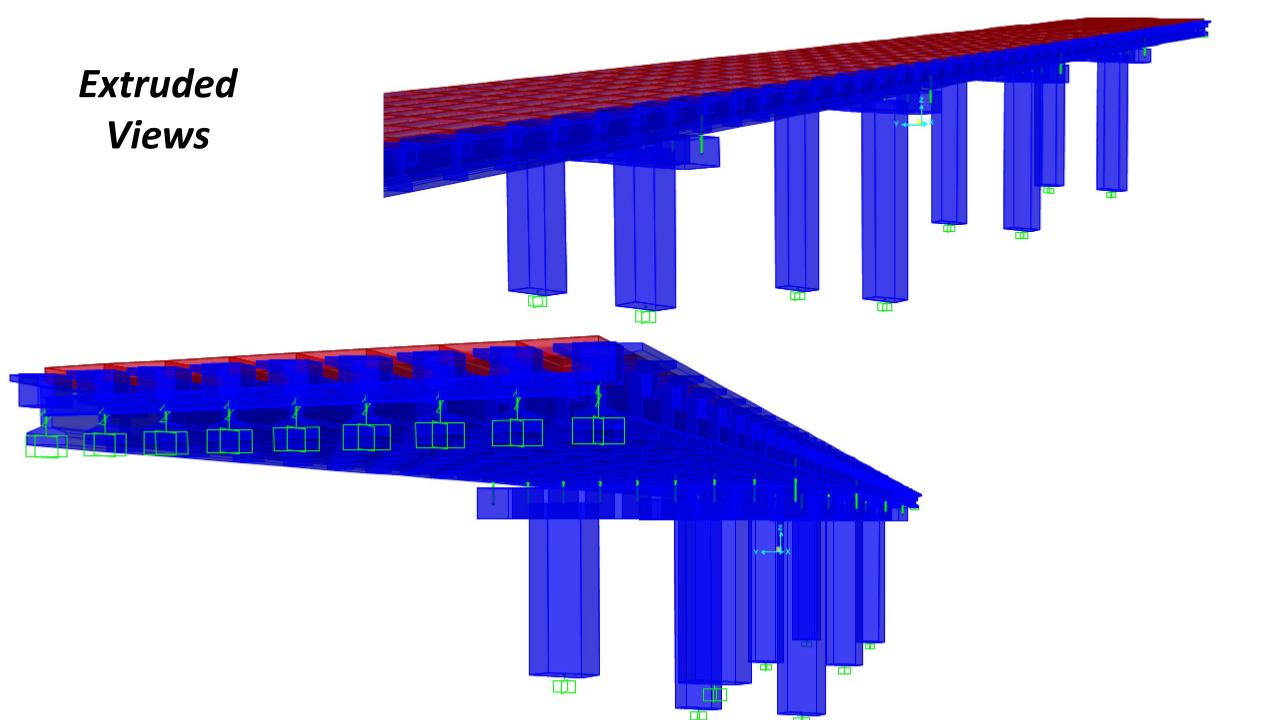
Bents are Skewed 22 degrees

Bridge Model – Shell Elements for the Cast-in-Place Concrete Deck



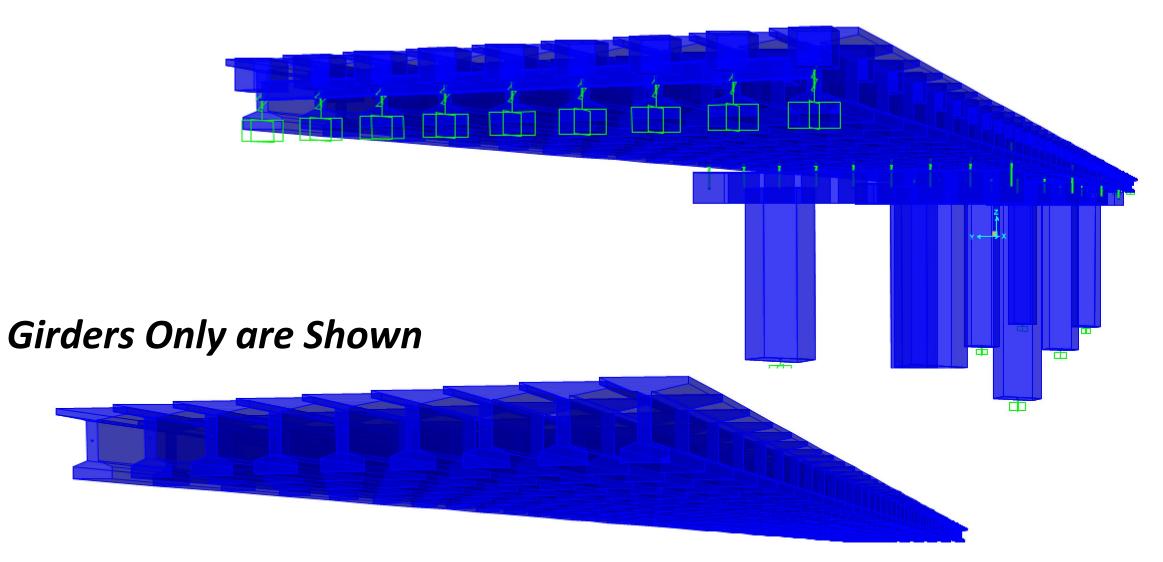
Bridge Model Details



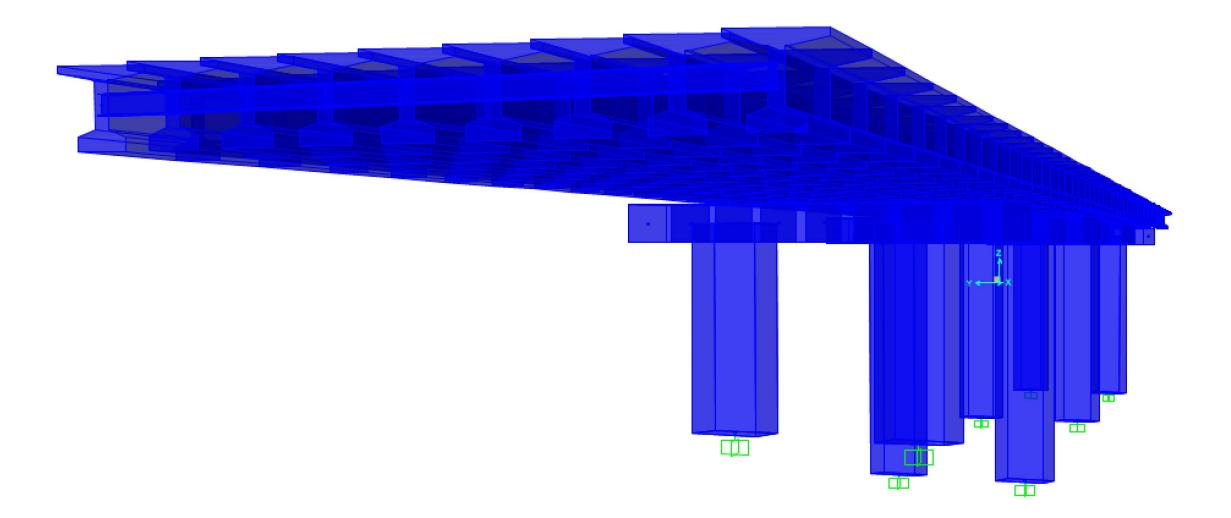


More Extruded Views

Deck not Shown



Extruded View, Girders, Bent Cap and Columns Shown



Conclusions (1/2)

- Bridges were heavily damaged but remained standing
- Tens of thousands of buildings collapsed
- M_w 7.8 earthquake is about M 8.1 on older Richter scale
- Turkiye earthquake was of similar size and type to 1906 San Francisco earthquake and the future "Big One" in California
- Fault rupture length was about the same distance as San Diego to Santa Barbara in California (with LA right between) or most of the length of Taiwan
- Shaking intensity is along the fault rupture line and not about epicenter

Conclusions (2/2)

- Unusual bridge damage of column plastic hinge part-way up cantilever column and degradation at precast girder ends
- On-going nonlinear bridge analysis with model as shown. Based on results from this global beam and shell model, a detailed 3D Finite Element model will be developed of just the girder end region
- Recommend physical testing in the laboratory of precast girder ends, including impact forces (dynamic) in all three directions
- Recommend physical testing in the laboratory of columns with different bar cutoffs